

KOFORMDEK 高鋼迪

Design To BS 5950-4/6

金屬模板 屋面鋼承板

Metal Decking & Roof Decking



建築金屬 圍護系統服務

SOLUTION Provider of Metal Building Envelope

KOFORMDEK 高鋼迪

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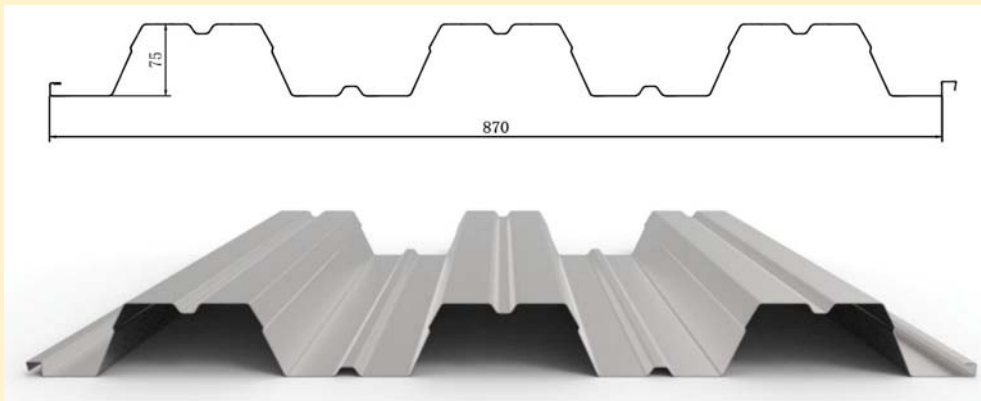
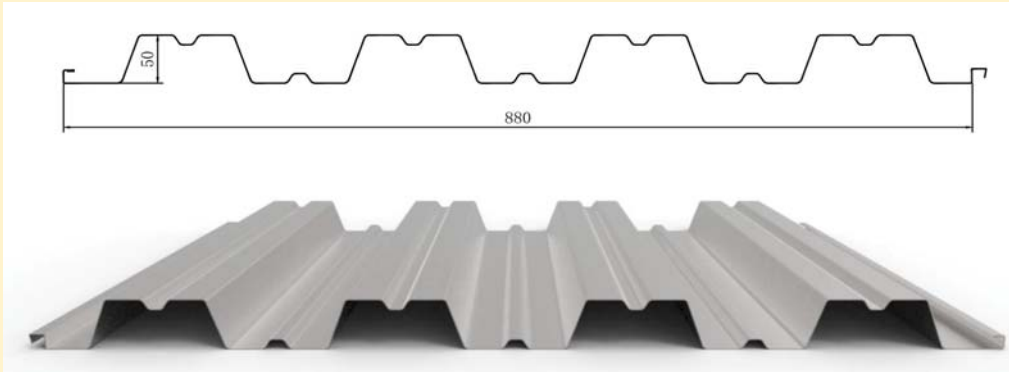
產品簡介

1. Products

產品簡介

1. Products

KOFORMDEK 高鋼迪常用規格:
KOFORMDEK Standard Specifications:



| 厚度 Thickness | 鍍層 Coating | 材質 Grade | 塗層 Colour - Optional |
|------------------|---------------|----------|----------------------|
| 0.75 / 1.0 / 1.2 | Z 275 / Z 350 | G550 | 髹塗 PE Coating |
| 1.5 | Z 275 / Z 350 | G450 | |



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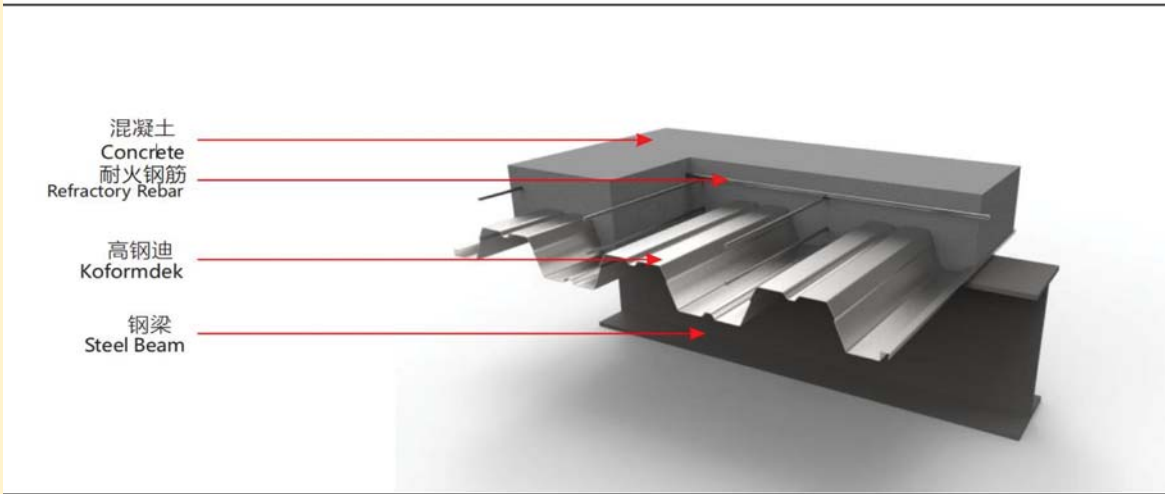
產品簡介

1. Products

作為金屬模板(樓承板)系統 Being Metal Decking System

KOFORMDEK 高鋼迪採用開口式的高板肋設計，高強承載力，強度比傳統の木模板更高，存儲壓型鋼承板較少會發生火災的機會。在短時間提供的工作平台，無需拆模，無需臨時支撐，可以多層面同時施工。產品獨特的搭接方式，確保混凝土不漏漿。新穎美觀的防滑花紋設計，與混凝土的結合更好。產品設計符合: GB、BS、EN、ASTM標準。

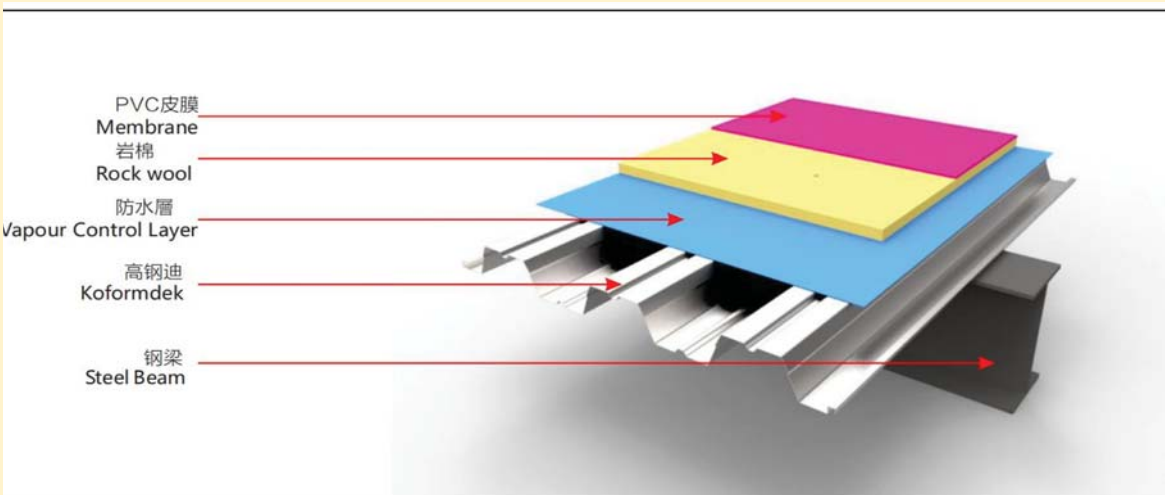
KOFORMDEK Metal deck plate adopts open high rib design, high strength bearing capacity, higher strength than traditional wood formwork, and there is less chance of fire when storing pressed steel plate. The working platform provided in a short period of time does not require demoulding or temporary support, and can be constructed at multiple levels at the same time. The unique lap joint method of the product ensures that the concrete does not leak. Novel and beautiful anti-skid pattern design, better combination with concrete. Product design conforms to: GB, BS, EN, ASTM standards.



作為屋面/天花系統 Being Roofing / Ceiling System

KOFORMDEK 高鋼迪鋼承板能應用於金屬屋面系統,可以減少結構支撐,增大室內使用空間,同時加快施工進度;鋼承板的塗層烤漆面可直接作為室內天花,省去天花吊頂;在鋼承板面上加上保溫岩棉和防水膜層,能組成結構性能優異的屋面系統。

KOFORMDEK steel deck can be applied to metal roofing systems, which can reduce structural support, increase indoor space, and speed up construction progress; The painted surface of the steel deck can be directly used as an indoor ceiling, eliminating the need for a ceiling; adding thermal insulation rock wool and a waterproof membrane layer on the steel deck can be combined into a roof system with excellent structural performance.



建築金屬 圍護系統服務

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設計說明

2 . Design

2.1 作為金屬模版(樓承板)系統 Being Metal Decking System

- ◆、壓型鋼承板的設計是遵照英標 BS5950：第6章：1995年“輕型壓型鋼承板設計實用規範”。

The design of the profiled steel decking is in accordance with BS5950:Part6:1995 ‘Code of practice for design of light gauge profiled steel sheeting’.

- ◆、複合樓板設計是遵照英標 BS5950：第四章：1994年“壓型鋼承板複合樓板設計實用規範”。

The design of the composite slabs with profiled steel decking is in accordance with BS5950:Part4:1994 ‘Code of practice for design of composite slabs with profiled steel sheeting’.

- ◆、壓型鋼承板截面性質和截面能力的測定均遵照英標 BS5950：第6章。

Section properties and determining the ability of pressure sectional steel carrier plate are in accordance with the British Standard BS5950: Chapter 6.

- ◆、在複合樓板的施工階段和被加強的混凝土板的凝固過程中壓型鋼樓板的設計荷載在有關荷載系數的設計荷載表中有介紹。

During the construction and concreting of reinforce concrete slabs, the design loads of the profiled steel decking are presented in Design Load Table with appropriate load factors.

- ◆、在計算壓型鋼承板的截面特性時需考慮以下兩個方面：

In calculating the pressure bearing plate steel section properties, two aspects should be considered:

- A. 壓力作用下翼緣的縱向彎曲以及彎曲作用下腹板的變形。

Under pressure flange buckling and bending deformation in webs.

- B. 壓型鋼承板在上撓彎矩作用下的塑性變形。

Pressure steel plate bearing on the flexible plastic deformation at the moment.

- ◆、對於單、雙和三跨的彎矩和剪力的設計值是根據已有的設計條件來計算的。對於在不同支撐下的變形係數也因此而得。

The design values of bending moment and shear forces for single span, double span and triple span are calculated in accordance with design information presented. The deflection coefficients for different support conditions are also employed accordingly.

- ◆、壓型鋼承板的模版跨距表是根據以下條件設計

彎矩抵抗模量、腹板抗剪能力、腹板抗壓模量、彎矩與壓力的複合作用、彎矩與剪力的複合作用、變形在樓板的施工階段及混凝土的凝結階段。

鋼承板段部支撐長度的最小值為 50mm。

The Formwork Load Span Table presented for profiled steel decking are designed against Moment resistance; Web shear capacity; Web crushing resistance; Combined bending and Web crushing; Combined bending and shear; and Deflection during the construction stage of composite slabs and also during the concreting of reinforced concrete slabs.

The minimum bearing length of end supports or end bearing is 50mm.

- ◆、壓型鋼承板的一般應用標準節點圖也在技術手冊《鋼承板跨度表》中介紹。

Typical details for common application of profiled steel decking are presented in technical manual Koforndek Formwork Load Span Tables.

設計說明

2 . Design

2.2 作為屋面/天花系統 Being Roofing / Ceiling System

- ◆、屋面壓型鋼板是根據英標BS5950：第6章：1995年“薄壁壓型鋼板的設計實行規範”。

The design of the profiled steel roof decking is in accordance with BS 5950:Part 6:1995 "Code of practice for design of light gauge profiled steel sheeting".

- ◆、壓型鋼承板的模板跨距表是根據以下條件設計

彎矩抵抗模量、腹板抗剪能力、腹板抗壓模量、彎矩與壓力的複合作用、彎矩與剪力的複合作用、變形在樓板的施工階段及混凝土的凝結階段。

The Formwork Load Span Table presented for profiled steel decking are designed against Moment resistance; Web shear capacity; Web crushing resistance; Combined bending and Web crushing; Combined bending and shear; and Deflection during the construction stage of composite slabs and also during the concreting of reinforced concrete slabs.

- ◆、屋面壓型鋼底板的橫截面的幾何形狀及材料已在之前的表格中有說明。

The cross section geometry and the material specifications of the profiled steel roof decking are presented.

- ◆、對於單、雙和三跨的彎矩和剪力的設計值是根據已有的設計條件來計算的。對於在不同支撐下的變形係數也因此而得。

The design values of bending moment and shear forces for single span, double span and triple span are calculated in accordance with design information. The deflection coefficients for different support conditions are also employed accordingly.

- ◆、在計算壓型鋼底板的截面特性時需考慮以下兩個方面：

In calculating the pressure bearing plate steel section properties, two aspects should be considered:

- A. 壓力作用下翼緣的縱向彎曲以及彎曲作用下腹板的變形。

Under pressure flange buckling and bending deformation in webs.

- B. 壓型鋼承板在上撓彎矩作用下的塑性變形。

Pressure steel plate bearing on the flexible plastic deformation at the moment.

- ◆、在每個波峯上至少打一顆螺釘，螺釘的連接力批Pt英標BS5950：第5章:1998年：附錄A中的條款A.1.5，螺釘的極限抗拉力Pft的最小值為8.4kN，該力使用了系數3。每顆螺釘的連接抗拉力Pft最大值為2.8kN。螺釘連接中的拉斷與拉脫都已考慮。螺釘和螺帽的最小直徑分別為4.83mm和10mm，而支撐板或標條的最小厚度為1.0mm。

In every rib, one screw is provided as minimum, and the tensile capacity of the screw connection, Pt, is calculated in accordance with clause A.1.5 of Appendix A:BS5950:Part:5:1998.The minimum tensile capacity of the screw, Pft, is 8.4kN(ultimate), and a factor of 3 is used; the maximum tensile capacity of each screwed connection .Pft , is 2.8kN,Both pulling-over and pulling-out of the screwed connections have also been considered. The minimum diameter of the screws and the washers are 4.83mm and 10mm respectively while the minimum thickness of supporting sheetings or purlin sections is 1.0mm.

設計材料

3. Materials

| 高鋼迪 KOFORMDEK | 鋼板厚度 (mm) Metal thickness | 屈服強度 Yield Strength Py(N/mm ²) | 鍍層 Coating | 可選配 Optional |
|------------------|------------------------------|---|---------------|---------------------------------|
| | | | | 鍍層 Coating / 烤漆 Paint |
| 50—880 75—870 | 0.75 | G450 | AZ185 Z275 | AZ185 / PE Z275 / SMP / PVDF |
| | 1.0 | | | |
| | 1.2 | G450 | Z275 | |
| | 1.5 | | Z350 | |

設計荷載

4. Design Load

4.1 施工階段的極限承載狀況和正常承載狀況 Construction stage—Ultimate limit state and Serviceability limit

| 類型 Type | 荷載係數 Load Factor | 設計值 Design value | |
|---------------------------|----------------------|--|--------------------------|
| 施工荷載 Construction load | 1.0(min) 1.6(max) | 最小 Minimum | 1.5 kN/m |
| | | If Lp < 3m (Lp is the span of decking) | 4.5/Lp kN/m ² |
| | | 最大 Maximum | 3.0 kN/m ² |
| 堆壓荷載 Storage load | 1.0(min) 1.6(max) | | 3.0 kN/m ² |
| 恒荷載 Dead load | 1.0(min) 1.4(max) | 鋼筋混凝土自重 Self weight of concrete | 24.0 kN/m ³ |
| | | 鋼承板自重 Self weight of decking | 78.5 kN/m ³ |

4.2 鋼承屋面板設計荷載 Design loads for roof decking

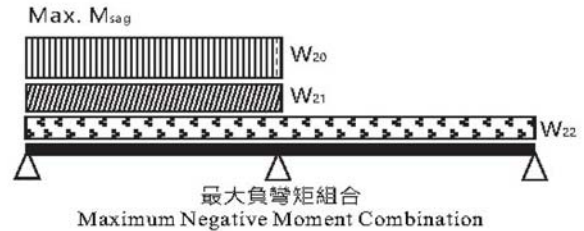
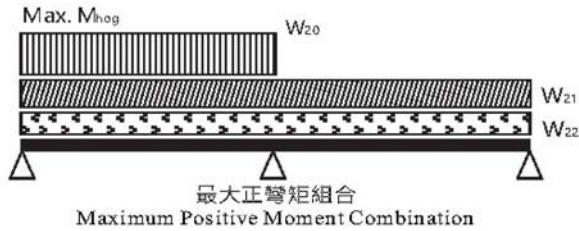
| 類型 Type | 荷載係數 Load Factor | 設計值 Design value |
|---------------------------|------------------|---|
| 風荷載 Wind load | 1.4(max) | Code of practice wind effects in Hong Kong 2004: 1.82kPa minimum |
| 活荷載 Imposed load | 1.6(max) | A concentrated load of 0.75kN (as an equivalent lin load of 0.75kN/m), Or a UDL of 0.75kN/m |
| 施工荷載 Construction load | 1.6(max) | A concentrated load of 0.75kN (as an equivalent lin load of 0.75kN/m) |

設計系數

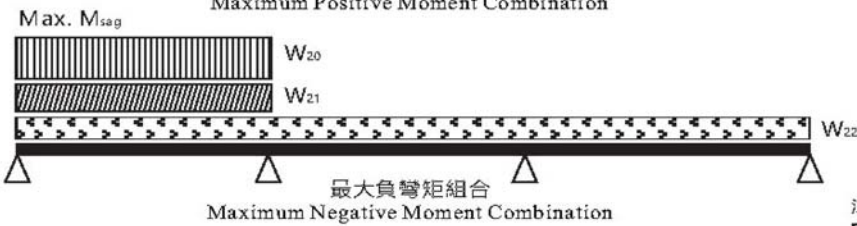
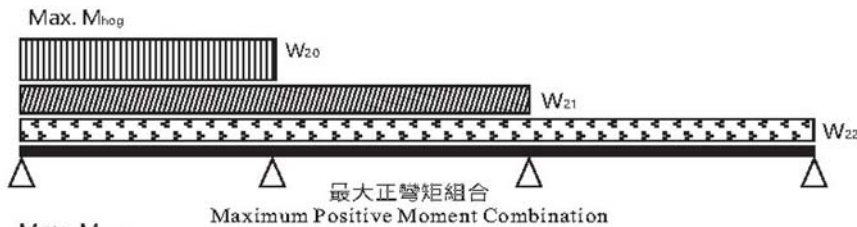
5 . Design Factor

設計系數

5 Design Factor






最大正彎矩 $\text{Max. } M_{\text{sag}} = 0.096W_{20} + 0.096W_{21} + 0.070W_{22}$
 最大負彎矩 $\text{Max. } M_{\text{hog}} = 0.063W_{20} + 0.125W_{21} + 0.125W_{22}$
 最大剪力 $\text{Max. } V = 0.562W_{20} + 0.625W_{21} + 0.625W_{22}$
 最大支座反力 $\text{Max. } R = 0.625W_{20} + 1.250W_{21} + 1.250W_{22}$



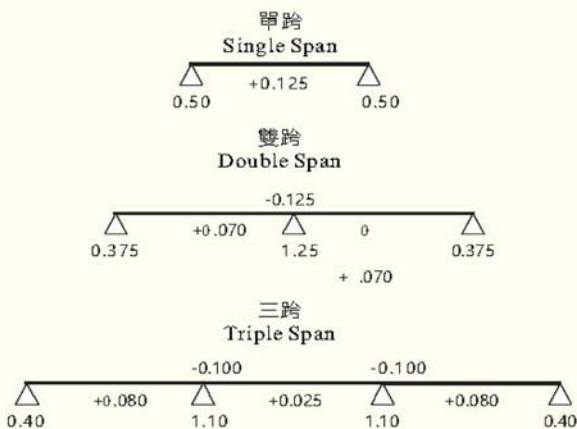
最大正彎矩 $\text{Max. } M_{\text{sag}} = 0.094W_{20} + 0.094W_{21} + 0.080W_{22}$
 最大負彎矩 $\text{Max. } M_{\text{hog}} = 0.067W_{20} + 0.117W_{21} + 0.100W_{22}$
 最大剪力 $\text{Max. } V = 0.567W_{20} + 0.617W_{21} + 0.600W_{22}$
 最大支座反力 $\text{Max. } R = 0.650W_{20} + 1.200W_{21} + 1.100W_{22}$

注釋 Notes:

-  $W_{20} = 1.6 \times 2/3 \text{ LL}_{\text{Cont}}$
-  $W_{21} = 1.4 \text{ DL}_{\text{conc}} + 1.6 \times 1/3 \text{ LL}_{\text{Cont}}$
-  $W_{22} = 1.4 \text{ DL}_{\text{deck}}$

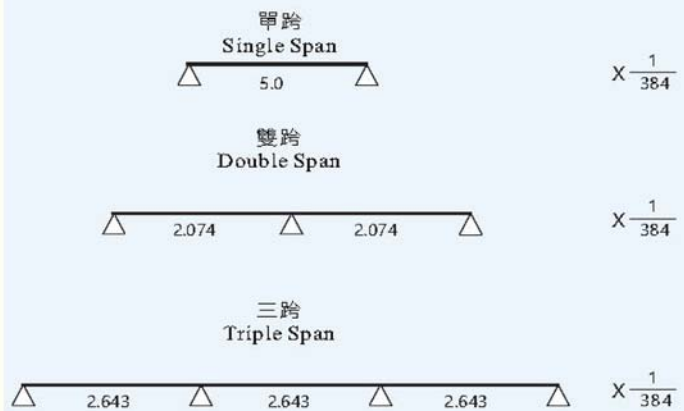
彎矩與支座反力系數

Bending Moment And Shear Force Coefficients



撓度係數

Deflection Coefficients



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性能介紹 6. Performance

6.1 KOFORMDEK 50-880 截面特性 KOFORMDEK 50-880 Section Properties

| Metal thickness (mm) | Weight (kg/m ²) | 設計強度 Design strength Py | 抗彎矩能力 moment capacity M _{xr} (kNm/m) | 次彎矩面積 Second moment of area | | |
|----------------------|-----------------------------|-------------------------|---|-------------------------------------|--------------------------------------|--------------------------------------|
| | | | | I _x (cm ⁴ /m) | I _{xr} (cm ⁴ /m) | I _{yv} (cm ⁴ /m) |
| 0.75 | 8.03 | (G450) | 4.86 | 46.00 | 33.48 | 39.74 |
| 1.0 | 10.70 | | 7.38 | 61.34 | 48.64 | 54.99 |
| 1.2 | 12.85 | G550 | 9.73 | 73.60 | 61.78 | 67.69 |
| 1.5 | 16.04 | G450 | 13.82 | 92.00 | 82.83 | 87.42 |

6.2 KOFORMDEK 高鋼迪50-880最大無支撐跨度表 Table of Maximum Unshored Deck Spans

| Slab Depth (mm) | 鋼板厚度 Thickness (mm) | Maximum Unshored Span | | |
|-----------------|---------------------|-----------------------|----------------|----------------|
| | | 單跨 Single Span | 雙跨 Double Span | 三跨 Triple Span |
| 100 | 0.75 | 2690 | 2760 | 3055 |
| | 1 | 3055 | 3595 | 3745 |
| | 1,20 | 3255 | 4195 | 3975 |
| 150 | 0.75 | 2260 | 2170 | 2425 |
| | 1 | 2660 | 2960 | 3250 |
| | 1,20 | 2840 | 3475 | 3490 |
| 200 | 0.75 | 1995 | 1825 | 2045 |
| | 1 | 2410 | 2500 | 2790 |
| | 1,20 | 2575 | 3015 | 3175 |
| 250 | 0.75 | 1810 | 1595 | 1785 |
| | 1 | 2235 | 2190 | 2450 |
| | 1,20 | 2390 | 2650 | 2950 |

Bearing Length=100mm

以上表中數據基於以下條件：
The above and below spans tables is based on following criteria

1. 混凝土比重為2400kg/m³
Unit weight of concrete 2400kg/m³
2. 施工時均布荷載：1kN/m²
Construction uniform load 1kN/m²
3. 撓度控制為：L/200與20mm的較小值
Deflection is limited to minimum of L/200 and 20mm

性能介紹 6 . Performance

高鋼迪50-880荷載與跨度使用表
6.3 KOFORMDEK 50-880 Load Span Table

| t=0.75mm | Limit State | | Span | | | | | | | |
|---------------|--------------------------------|---------------------------|-------|-------|------|------|------|------|------|------|
| | | | 2500 | 3000 | 3500 | 4000 | 4500 | 5000 | 5500 | 6000 |
| Single span | Ultimate-factored load | One screw per trough | 6.22 | 4.31 | 3.17 | 2.43 | 1.92 | 1.55 | 1.28 | -/- |
| | | Two screws per trough | 6.22 | 4.31 | 3.17 | 2.43 | 1.92 | 1.55 | 1.28 | -/- |
| | Serviceability-unfactored load | Deflection limit=span/200 | 1.69 | 0.98 | 0.61 | 0.41 | 0.29 | 0.21 | 0.16 | -/- |
| | | Deflection limit=span/90 | 3.75 | 2.17 | 1.37 | 0.92 | 0.64 | 0.47 | 0.35 | -/- |
| End span | Ultimate-factored load | One screw per trough | 3.33 | 2.77 | 2.38 | 2.08 | 1.85 | 1.55 | 1.28 | -/- |
| | | Two screws per trough | 6.16 | 4.29 | 3.15 | 2.42 | 1.91 | 1.55 | 1.28 | -/- |
| | Serviceability-unfactored load | Deflection limit=span/200 | 3.18 | 1.84 | 1.16 | 0.78 | 0.55 | 0.40 | 0.30 | -/- |
| | | Deflection limit=span/90 | 7.07 | 4.09 | 2.58 | 1.73 | 1.21 | 0.88 | 0.66 | -/- |
| Internal span | Ultimate-factored load | One screw per trough | 3.20 | 2.67 | 2.28 | 2.00 | 1.78 | 1.60 | 1.45 | 1.27 |
| | | Two screws per trough | 6.41 | 5.04 | 3.71 | 2.85 | 2.25 | 1.82 | 1.51 | 1.27 |
| | Serviceability-unfactored load | Deflection limit=span/200 | 8.43 | 4.88 | 3.07 | 2.06 | 1.45 | 1.05 | 0.79 | 0.61 |
| | | Deflection limit=span/90 | 15.00 | 10.85 | 6.83 | 4.58 | 3.21 | 2.34 | 1.76 | 1.36 |

| t=1.0 mm | Limit State | | Span | | | | | | | |
|---------------|--------------------------------|---------------------------|-------|-------|------|------|------|------|------|------|
| | | | 2500 | 3000 | 3500 | 4000 | 4500 | 5000 | 5500 | 6000 |
| Single span | Ultimate-factored load | One screw per trough | 8.33 | 6.55 | 4.81 | 3.69 | 2.91 | 2.36 | 1.95 | 1.64 |
| | | Two screws per trough | 9.44 | 6.55 | 4.81 | 3.69 | 2.91 | 2.36 | 1.95 | 1.64 |
| | Serviceability-unfactored load | Deflection limit=span/200 | 2.45 | 1.42 | 0.89 | 0.60 | 0.42 | 0.31 | 0.23 | 0.18 |
| | | Deflection limit=span/90 | 5.45 | 3.15 | 1.98 | 1.33 | 0.93 | 0.68 | 0.51 | 0.39 |
| End span | Ultimate-factored load | One screw per trough | 3.33 | 2.77 | 2.38 | 2.08 | 1.85 | 1.66 | 1.51 | 1.38 |
| | | Two screws per trough | 6.66 | 5.55 | 4.76 | 3.68 | 2.90 | 2.35 | 1.94 | 1.63 |
| | Serviceability-unfactored load | Deflection limit=span/200 | 4.62 | 2.68 | 1.68 | 1.13 | 0.79 | 0.58 | 0.43 | 0.33 |
| | | Deflection limit=span/90 | 10.27 | 5.59 | 3.74 | 2.51 | 1.76 | 1.28 | 0.96 | 0.74 |
| Internal span | Ultimate-factored load | One screw per trough | 3.20 | 2.67 | 2.28 | 2.00 | 1.78 | 1.60 | 1.45 | 1.33 |
| | | Two screws per trough | 6.41 | 5.34 | 4.57 | 4.00 | 3.42 | 2.77 | 3.39 | 1.93 |
| | Serviceability-unfactored load | Deflection limit=span/200 | 12.25 | 7.09 | 4.47 | 2.99 | 2.10 | 1.53 | 1.15 | 0.89 |
| | | Deflection limit=span/90 | 15.00 | 15.00 | 9.92 | 6.65 | 4.67 | 3.40 | 2.56 | 1.97 |

| t=1.2 mm | Limit State | | Span | | | | | | | |
|---------------|--------------------------------|---------------------------|-------|-------|-------|------|------|------|------|------|
| | | | 2500 | 3000 | 3500 | 4000 | 4500 | 5000 | 5500 | 6000 |
| Single span | Ultimate-factored load | One screw per trough | 8.33 | 6.94 | 5.95 | 4.68 | 3.70 | 2.99 | 2.47 | 2.08 |
| | | Two screws per trough | 11.99 | 8.32 | 6.11 | 4.68 | 3.70 | 2.77 | 2.47 | 2.08 |
| | Serviceability-unfactored load | Deflection limit=span/200 | 3.11 | 1.80 | 1.13 | 0.76 | 0.53 | 0.39 | 0.29 | 0.23 |
| | | Deflection limit=span/90 | 6.92 | 4.00 | 2.52 | 1.69 | 1.19 | 0.86 | 0.65 | 0.50 |
| End span | Ultimate-factored load | One screw per trough | 3.33 | 2.77 | 2.38 | 2.08 | 1.85 | 1.66 | 1.51 | 1.38 |
| | | Two screws per trough | 6.66 | 5.55 | 4.76 | 4.16 | 3.69 | 2.99 | 2.47 | 2.08 |
| | Serviceability-unfactored load | Deflection limit=span/200 | 5.87 | 3.40 | 2.14 | 1.43 | 1.01 | 0.73 | 0.55 | 0.42 |
| | | Deflection limit=span/90 | 13.05 | 7.55 | 4.76 | 3.19 | 2.24 | 1.63 | 1.23 | 0.94 |
| Internal span | Ultimate-factored load | One screw per trough | 3.20 | 3.67 | 2.28 | 2.00 | 1.78 | 1.60 | 1.45 | 1.33 |
| | | Two screws per trough | 6.41 | 5.34 | 4.57 | 4.00 | 3.56 | 3.20 | 2.91 | 2.45 |
| | Serviceability-unfactored load | Deflection limit=span/200 | 15.00 | 9.01 | 5.67 | 3.80 | 2.67 | 1.95 | 1.46 | 1.13 |
| | | Deflection limit=span/90 | 15.00 | 15.00 | 12.60 | 8.44 | 5.93 | 4.32 | 3.25 | 2.50 |

Notes: $P_y=550N/mm$, Bearing length=60mm, Unit=kPa

性能介紹 6. Performance

高鋼迪 75-870 截面特性

6.4 KOFORMDEK 75-870 Section Properties

| Metal thickness (mm) | Weight (kg/m ²) | 設計強度 Design strength Py | 抗彎矩能力 moment capacity M _{xr} (kNm/m) | 次彎矩面積 Second moment of area | | |
|----------------------|-----------------------------|-------------------------------|--|--|---|--------------------------------------|
| | | | | I _x (cm ⁴ /m) | I _{xr} (cm ⁴ /m) | I _{av} (cm ⁴ /m) |
| | | | | 0.75 | 8.12 | (G450) |
| 1.0 | 10.83 | 9.82 | 139.46 | 105.23 | 122.34 | |
| 1.2 | 12.99 | G550 | 13.61 | 167.35 | 132.78 | 150.06 |
| 1.5 | 12.99 | G450 | 18.84 | 209.18 | 177.04 | 193.11 |

高鋼迪 75-870 最大無支撐跨度表

6.5 KOFORMDEK Table of Maximum Unshored Deck Spans

| Slab Depth (mm) | 鋼板厚度 Thickness (mm) | Maximum Unshored Span | | |
|-----------------|------------------------|-----------------------|----------------|----------------|
| | | 單跨 Single Span | 雙跨 Double Span | 三跨 Triple Span |
| 100 | 0.75 | 3005 | 2990 | 3245 |
| | 1 | 3980 | 4115 | 4460 |
| | 1,20 | 4280 | 4945 | 5020 |
| 150 | 0.75 | 2560 | 2235 | 2505 |
| | 1 | 3505 | 3295 | 3610 |
| | 1,20 | 3375 | 3980 | 4365 |
| 200 | 0.75 | 2230 | 1825 | 2050 |
| | 1 | 3110 | 2750 | 3070 |
| | 1,20 | 3410 | 3385 | 3730 |
| 250 | 0.75 | 2010 | 1560 | 1755 |
| | 1 | 2800 | 2360 | 2650 |
| | 1,20 | 3155 | 2965 | 3285 |

Bearing Length=100mm

以上表中數據基於以下條件：

The above and below spans tables is based on following criteria

1. 混凝土比重為2400kg/m³
Unit weight of concrete 2400kg/m³
2. 施工時均布荷載：1kN/m²
Construction uniform load 1kN/m²
3. 撓度控制為：L/200與20mm的較小值
Deflection is limited to minimum of L/200 and 20mm

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性能介紹 6 . Performance

高鋼迪75-870荷載與跨度使用表
6.6 KOFORMDEK 75-870 Load Span Table

| t=0.75mm | Limit State | | Span | | | | | | | |
|---------------|--------------------------------|---------------------------|-------|-------|-------|------|------|------|------|------|
| | | | 2500 | 3000 | 3500 | 4000 | 4500 | 5000 | 5500 | 6000 |
| Single span | Ultimate-factored load | One screw per trough | 7.11 | 4.94 | 3.63 | 2.78 | 2.19 | 1.77 | 1.47 | 1.23 |
| | | Two screws per trough | 7.11 | 4.94 | 3.63 | 2.78 | 2.19 | 1.77 | 1.47 | 1.23 |
| | Serviceability-unfactored load | Deflection limit=span/200 | 3.53 | 2.05 | 1.29 | 0.86 | 0.61 | 0.44 | 0.33 | 0.26 |
| | | Deflection limit=span/90 | 7.85 | 4.55 | 2.86 | 1.92 | 1.35 | 0.98 | 0.74 | 0.57 |
| End span | Ultimate-factored load | One screw per trough | 4.96 | 4.13 | 3.54 | 2.76 | 2.18 | 1.77 | 1.46 | 1.23 |
| | | Two screws per trough | 7.03 | 4.90 | 3.61 | 2.76 | 2.18 | 1.77 | 1.46 | 1.23 |
| | Serviceability-unfactored load | Deflection limit=span/200 | 6.67 | 3.86 | 2.43 | 1.63 | 1.14 | 0.83 | 0.63 | 0.48 |
| | | Deflection limit=span/90 | 14.82 | 8.58 | 5.40 | 3.62 | 2.54 | 1.85 | 1.39 | 1.07 |
| Internal span | Ultimate-factored load | One screw per trough | 4.76 | 3.97 | 3.40 | 2.98 | 2.57 | 2.08 | 1.72 | 1.45 |
| | | Two screws per trough | 8.25 | 5.76 | 4.24 | 3.25 | 2.57 | 2.08 | 1.72 | 1.45 |
| | Serviceability-unfactored load | Deflection limit=span/200 | 15.00 | 10.23 | 6.44 | 4.31 | 3.03 | 2.21 | 1.66 | 1.28 |
| | | Deflection limit=span/90 | 15.00 | 15.00 | 14.31 | 9.59 | 6.73 | 4.91 | 3.69 | 2.84 |

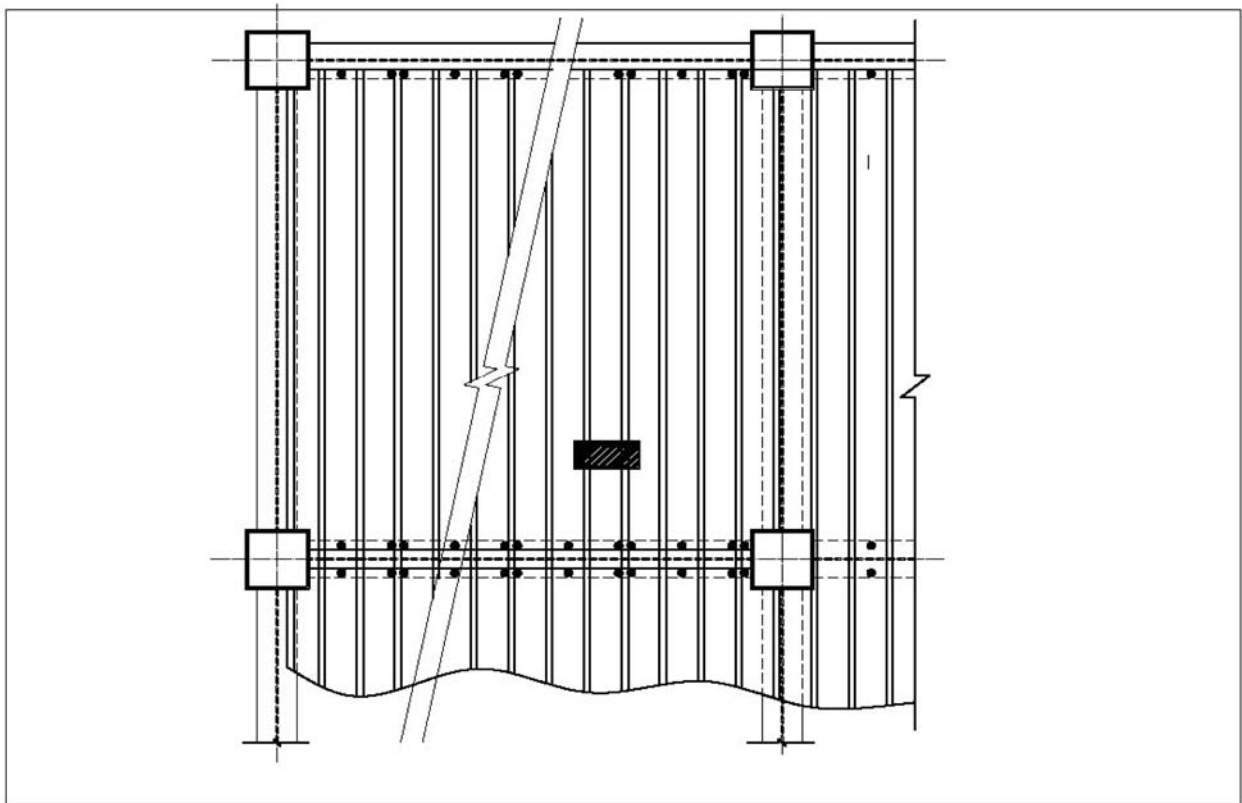
| t=1.0 mm | Limit State | | Span | | | | | | | |
|---------------|--------------------------------|---------------------------|-------|-------|-------|-------|-------|------|------|------|
| | | | 2500 | 3000 | 3500 | 4000 | 4500 | 5000 | 5500 | 6000 |
| Single span | Ultimate-factored load | One screw per trough | 12.40 | 8.72 | 6.41 | 4.90 | 3.87 | 3.14 | 2.59 | 2.18 |
| | | Two screws per trough | 12.56 | 8.72 | 6.41 | 4.90 | 3.87 | 3.14 | 2.59 | 2.18 |
| | Serviceability-unfactored load | Deflection limit=span/200 | 5.30 | 3.07 | 1.93 | 1.29 | 0.91 | 0.66 | 0.50 | 0.38 |
| | | Deflection limit=span/90 | 11.78 | 6.82 | 4.29 | 2.88 | 2.02 | 1.47 | 1.11 | 0.85 |
| End span | Ultimate-factored load | One screw per trough | 4.96 | 4.13 | 3.54 | 3.10 | 2.75 | 2.48 | 2.25 | 2.06 |
| | | Two screws per trough | 9.92 | 8.26 | 6.39 | 4.89 | 3.87 | 3.13 | 2.59 | 2.17 |
| | Serviceability-unfactored load | Deflection limit=span/200 | 10.00 | 5.79 | 3.65 | 2.44 | 1.72 | 1.25 | 0.94 | 0.72 |
| | | Deflection limit=span/90 | 15.00 | 12.86 | 8.10 | 5.43 | 3.81 | 2.78 | 2.09 | 1.61 |
| Internal span | Ultimate-factored load | One screw per trough | 4.76 | 3.97 | 3.40 | 2.98 | 2.64 | 2.38 | 2.16 | 1.98 |
| | | Two screws per trough | 9.53 | 7.94 | 6.81 | 5.76 | 4.56 | 3.69 | 3.05 | 2.56 |
| | Serviceability-unfactored load | Deflection limit=span/200 | 15.00 | 15.00 | 9.66 | 6.47 | 4.55 | 3.31 | 2.49 | 1.92 |
| | | Deflection limit=span/90 | 15.00 | 15.00 | 15.00 | 14.38 | 10.10 | 7.36 | 5.53 | 4.26 |

| t=1.2 mm | Limit State | | Span | | | | | | | |
|---------------|--------------------------------|---------------------------|-------|-------|-------|-------|-------|------|------|------|
| | | | 2500 | 3000 | 3500 | 4000 | 4500 | 5000 | 5500 | 6000 |
| Single span | Ultimate-factored load | One screw per trough | 12.40 | 10.33 | 8.85 | 6.80 | 5.37 | 4.35 | 3.59 | 3.02 |
| | | Two screws per trough | 17.42 | 12.09 | 8.88 | 6.80 | 5.37 | 4.35 | 3.59 | 3.02 |
| | Serviceability-unfactored load | Deflection limit=span/200 | 6.69 | 3.87 | 2.44 | 1.63 | 1.15 | 0.84 | 0.63 | 0.48 |
| | | Deflection limit=span/90 | 14.86 | 8.60 | 5.42 | 3.63 | 2.55 | 1.86 | 1.40 | 1.08 |
| End span | Ultimate-factored load | One screw per trough | 4.96 | 4.13 | 3.54 | 3.10 | 2.75 | 2.48 | 2.25 | 2.06 |
| | | Two screws per trough | 9.92 | 8.26 | 7.08 | 6.20 | 5.36 | 4.34 | 3.59 | 3.02 |
| | Serviceability-unfactored load | Deflection limit=span/200 | 12.62 | 7.30 | 4.60 | 3.08 | 2.16 | 1.58 | 1.19 | 0.91 |
| | | Deflection limit=span/90 | 15.00 | 15.00 | 10.22 | 6.85 | 4.81 | 3.51 | 2.63 | 2.03 |
| Internal span | Ultimate-factored load | One screw per trough | 4.76 | 3.97 | 3.40 | 2.98 | 2.64 | 2.38 | 2.16 | 1.98 |
| | | Two screws per trough | 9.53 | 7.94 | 6.18 | 5.96 | 5.29 | 4.76 | 4.23 | 3.56 |
| | Serviceability-unfactored load | Deflection limit=span/200 | 15.00 | 15.00 | 12.19 | 8.17 | 5.73 | 4.18 | 3.14 | 2.42 |
| | | Deflection limit=span/90 | 15.00 | 15.00 | 15.00 | 15.00 | 12.74 | 9.29 | 6.98 | 5.38 |

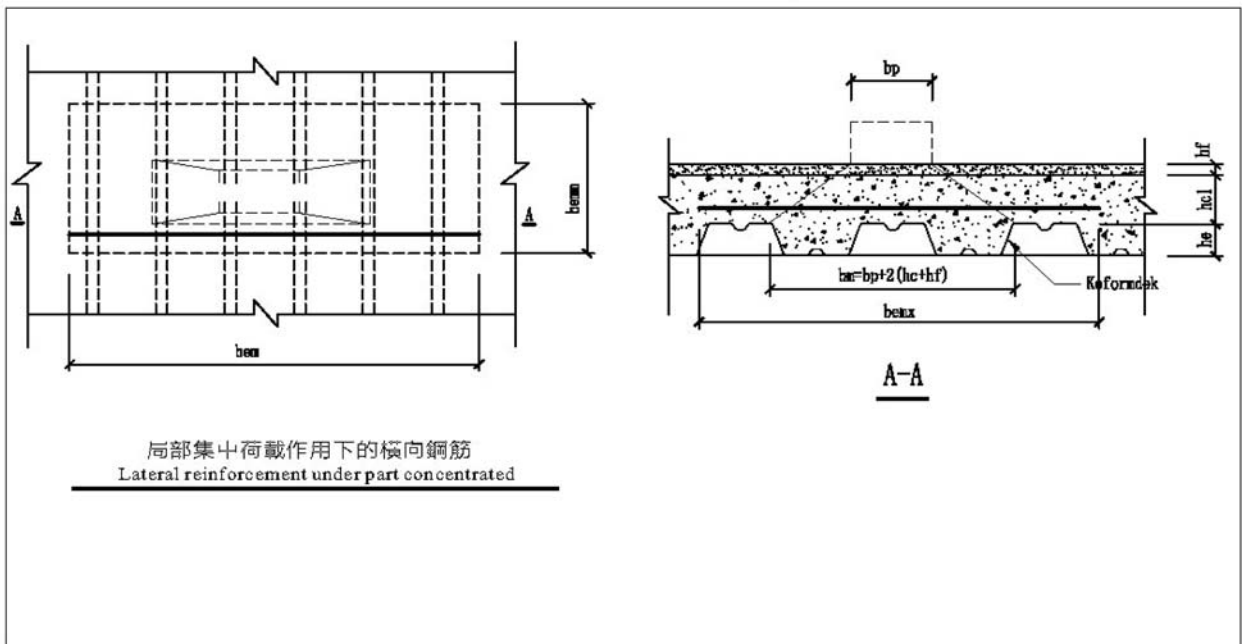
Notes: Py=550N/mm , Bearing length=60mm, Unit=kPa

典型節點 7. Typical Details

7. Typical Details



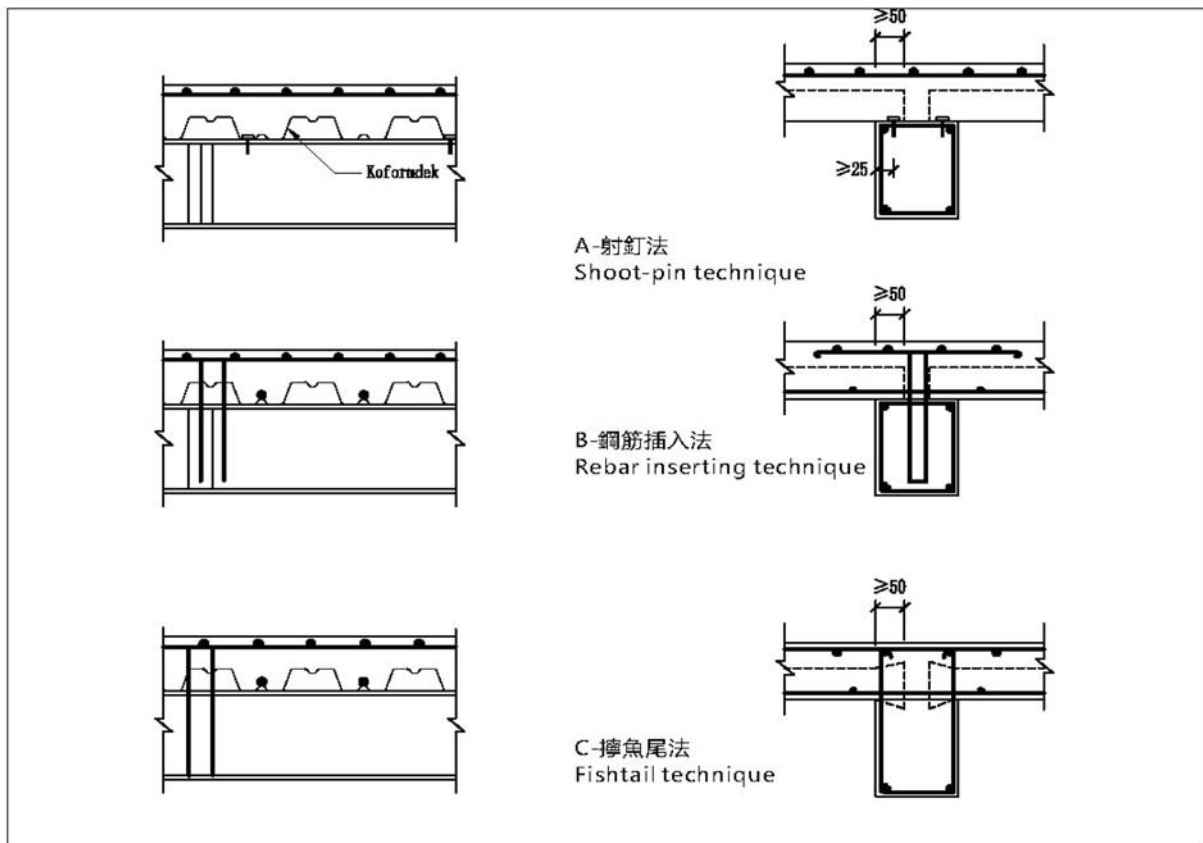
壓型鋼板鋪板平面示意圖
Profiled steel Layout



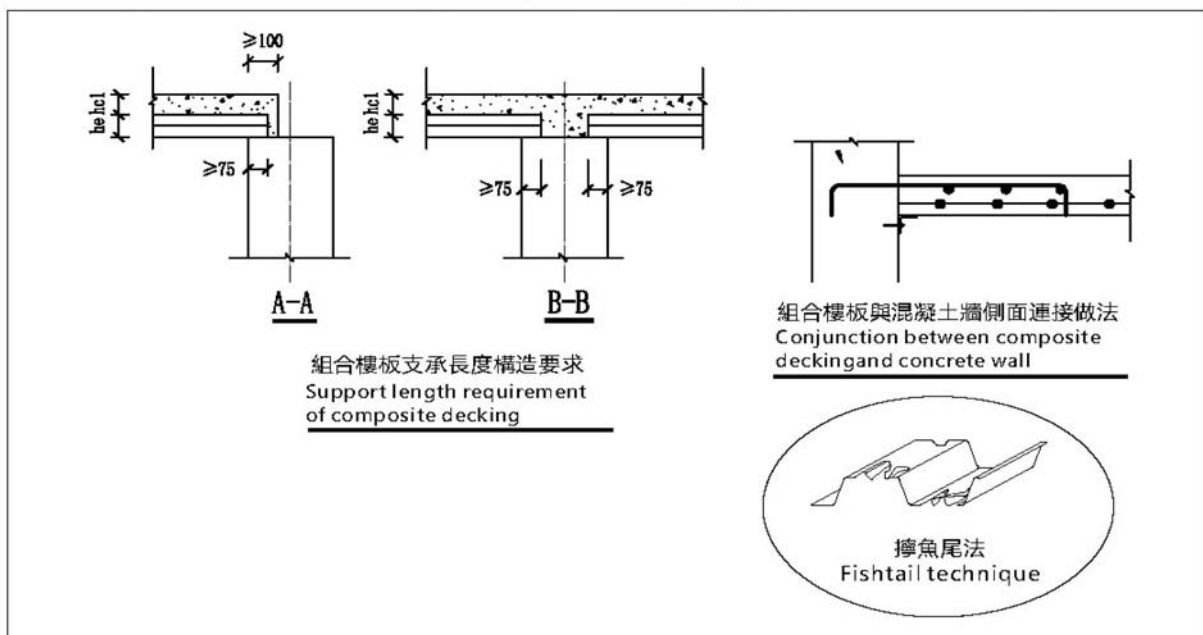
局部集中荷載作用下的橫向鋼筋
Lateral reinforcement under part concentrated

典型節點 7. Typical Details

典型節點 7. Typical Details



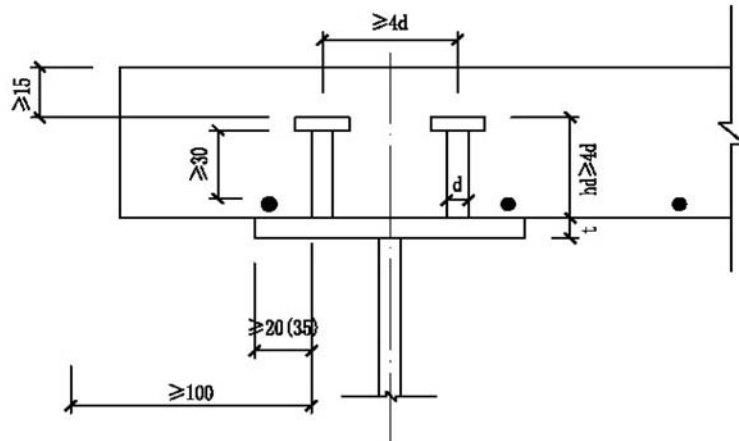
壓型鋼板錨固方法
Fishtail technique



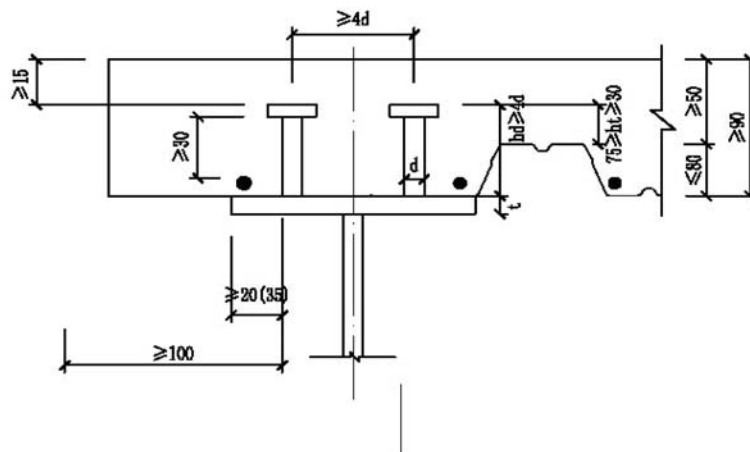
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典型節點 7. Typical Details

典型節點 7. Typical Details



a. 組合梁翼板為現澆混凝土平板
The composite beam flange is cast-in-place concrete



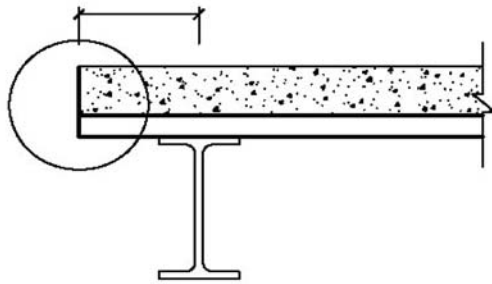
b. 組合梁翼板為壓型鋼板-混凝土組合樓板
The composite beam flange is steel-concrete concrete decking

栓釘抗剪連接件構造(垂直梁長度方向)
Shear studs connection structure (vertical beam direction)

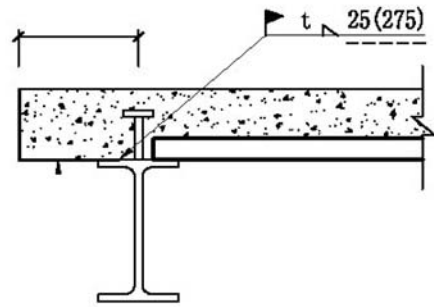
典型節點 7. Typical Details

典型節點

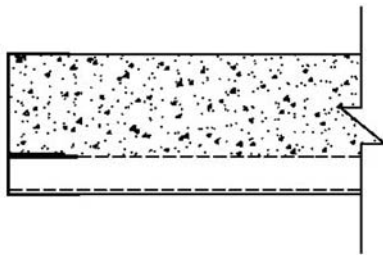
7. Typical Details



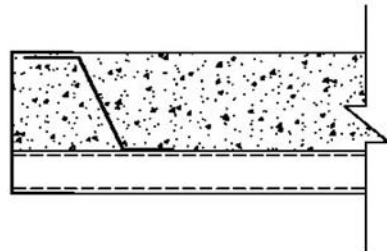
板肋與梁垂直收邊構造(1)
Plate ribs and beams perpendicular
to the edge of the structure (1)



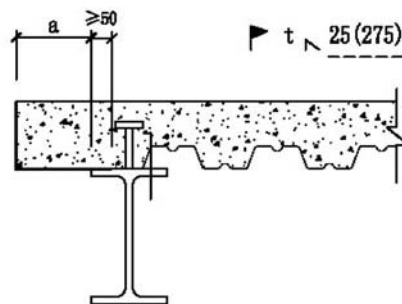
板肋與梁垂直收邊構造(2)
Plate ribs and beams perpendicular
to the edge of the structure (2)



橫向收邊(1)
Horizontal Edge (1)



橫向收邊(2)
Horizontal Edge (2)

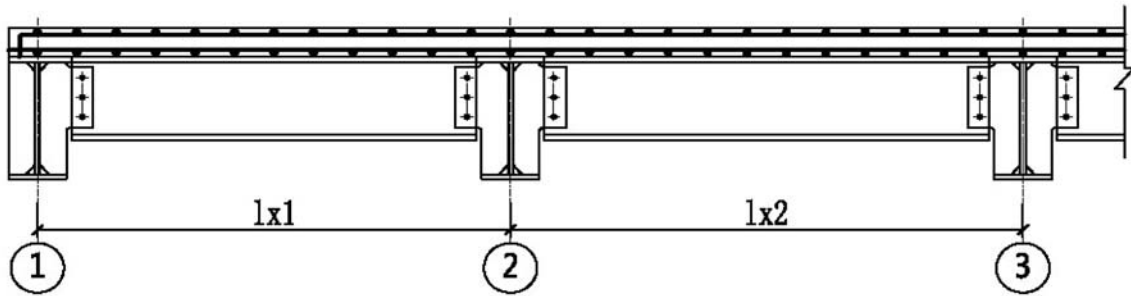


板肋與梁平行收邊構造
Parallel Structure of Plate Rib and Beam
($a \leq 250$)

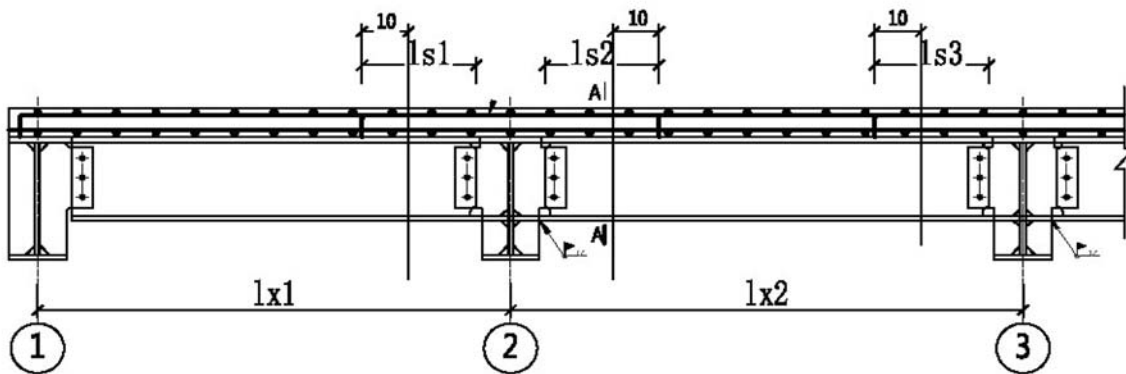
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典型節點
7. Typical Details

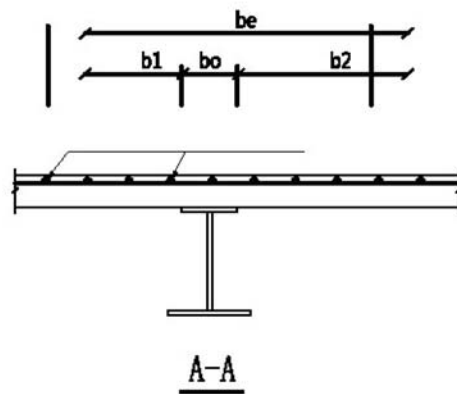
典型節點
7. Typical Details



1-1 簡支組合梁節點連接及配筋構造
Joint connection of simply supported composite beam and reinforcement structure



2-2 連續組合梁節點連接及配筋構造
Connection of connecting composite beams and reinforcement structure



工程案例
8 . Project Reference

工程案例
8 . Project Reference



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